Skye Area Plan

Amendment #2

SEWER SECTION

July 27, 2023

Prepared By:



3302 N Main Street Spanish Fork, UT 84660 (801) 798-0555

Sewer

Design Criteria:

Minimum Pipe Size: 8" Minimum Slope: 0.34%

Design Flow:

Residential 100 gallons per person per day

(UT R317-3-2.2.B.1)

3.8 capita / Dwelling Unit

Mixed Use 827 ERUs allocated with Land Use Plan

3.8 capita / ERU

100 gpd / capita

Civic 29 ERUs allocated with Land Use

3.8 capita / ERU 100 gpd / capita

School 24 ERUs allocated with Land Use Plan

3.8 capita / ERU 100 gpd / capita

Peak Factor 3.0

Refer to Appendix A for an exhibit showing each pipe location and Appendix B for a detailed breakdown of sewer flows and pipe sizing.

Allowable Flows:

West Side:

The original Micron area plan called for an allowable discharge of 1,851 gallons per minute (gpm) into the existing sewer system in 500 W, as well as 62 gpm into the existing outfall near 600 E. The new sewer design will remain consistent with this area plan amendment. 62 gpm will be allowed to flow into the 600 E outfall, while the rest of the west side flow will be routed to the 500 W outfall, including the flow from the additional 300 units from the upper area allowed by the city council. The overall total projected peak flow into this outfall is expected to be 2,054 gpm.

East Side:

The maximum allowable flow on the east side of Skye was determined based on an agreement between Lehi City and the Timpanogos Special Service District (TSSD). This agreement states that a maximum average flow of 227,000 gallons per day (GPD) is allowed to flow into the existing sewer system. Using a peaking factor of 3, this converts to approximately 473 gpm, or in other words, 597 ERU's. The upper area being added to this area plan is sloped such that some of it will need to sewer down to the east side. This area plan amendment proposes to discharge the flow from approximately 621 ERU's (492 gpm) as approved by TSSD.

Methodology:

All pipes were designed according to the Utah administrative Rules as listed below:

R317-3-2. Sewers.

2.1. General. Construction of a new sewer system project may not begin unless the applicant has submitted an engineering report detailing the design, and construction plans to the executive secretary for review and approval evidenced by a construction permit. The executive secretary will not normally review construction plans for extensions of the existing sewer systems to new areas or replacement of sanitary sewers in the existing sewer systems unless requested or required by state or federal funding programs. Rain water from roofs, streets, and other areas, and ground water from foundation drains must not be allowed to enter the sewer system through planning, design and construction quality assurance and control measures.

2.2. Basis of Design

- A. Planning Period. Sewers should be designed for the estimated ultimate tributary population or the 50-year planning period, whichever requires a larger capacity. The executive secretary may approve the design for reduced capacities provided the capacity of the system can be readily increased when required. The maximum anticipated capacity required by institutions, industrial parks, etc. must be considered in the design.
- B. Sewer Capacity. The required sewer capacity shall be determined on the basis of maximum hourly domestic sewage flow; additional maximum flow from industrial plants; inflow; ground water infiltration; potential for sulfide generation; topography of area; location of sewage treatment plant; depth of excavation; and pumping requirements.
- 1. Per Capita Flow. New sewer systems shall be designed on the basis of an annual average daily rate of flow of 100 gallons per capita per day (0.38 cubic meter per capita per day) unless there are data to indicate otherwise. The per capita rate of flow includes an allowance for infiltration/inflow. The per capita rate of flow may be higher than 100 gallons per day (0.38 cubic meter per day) if there is a probability of large amounts of infiltration/inflow entering the system.

2. Design Flow

- a. Laterals and collector sewers shall be designed for 400 gallons per capita per day (1.51 cubic meters per capita per day).
- b. Interceptors and outfall sewers shall be designed for 250 gallons per capita per day (0.95 cubic meter per capita per day), or rates of flow established from an approved infiltration/inflow study.
- c. The executive secretary will consider other rates of flow for the design if such basis is justified on the basis of supporting documentation.

C. Design Calculations. Detailed computations, such as the basis of design and hydraulic calculations showing depth of flow, velocity, water surface profiles, and gradients shall be submitted with plans.

2.3. Design and Construction Details

A. Minimum Size

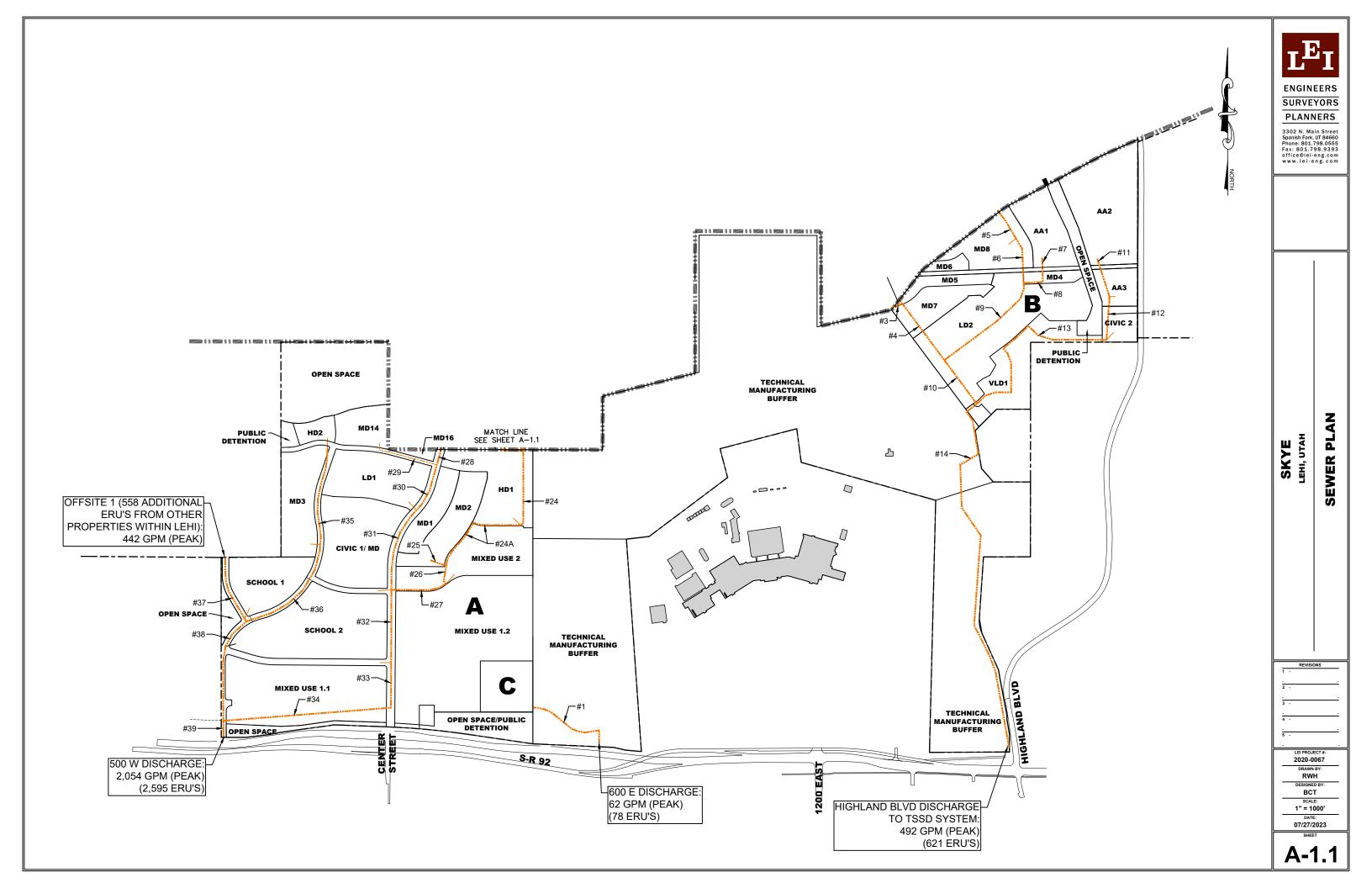
- 1. No gravity sewer shall be of less than eight inches (20 centimeters) in diameter.
- 2. A 6-inch (15 centimeters) diameter pipe may be permitted when the sewer is serving only one connection, or if the applicant justifies the need for such diameter on the basis of supporting documentation.
- B. Depth. Sewers should be sufficiently deep to receive sewage from basements and to prevent freezing. Insulation shall be provided for sewers that cannot be placed at a depth sufficient to prevent freezing.
- C. Odor and Sulfide Generation. The design shall incorporate features to control and mitigate odor and sulfide generation in sewers. Such features may include steeper slope to achieve higher velocity, reaeration through induced turbulence, etc.

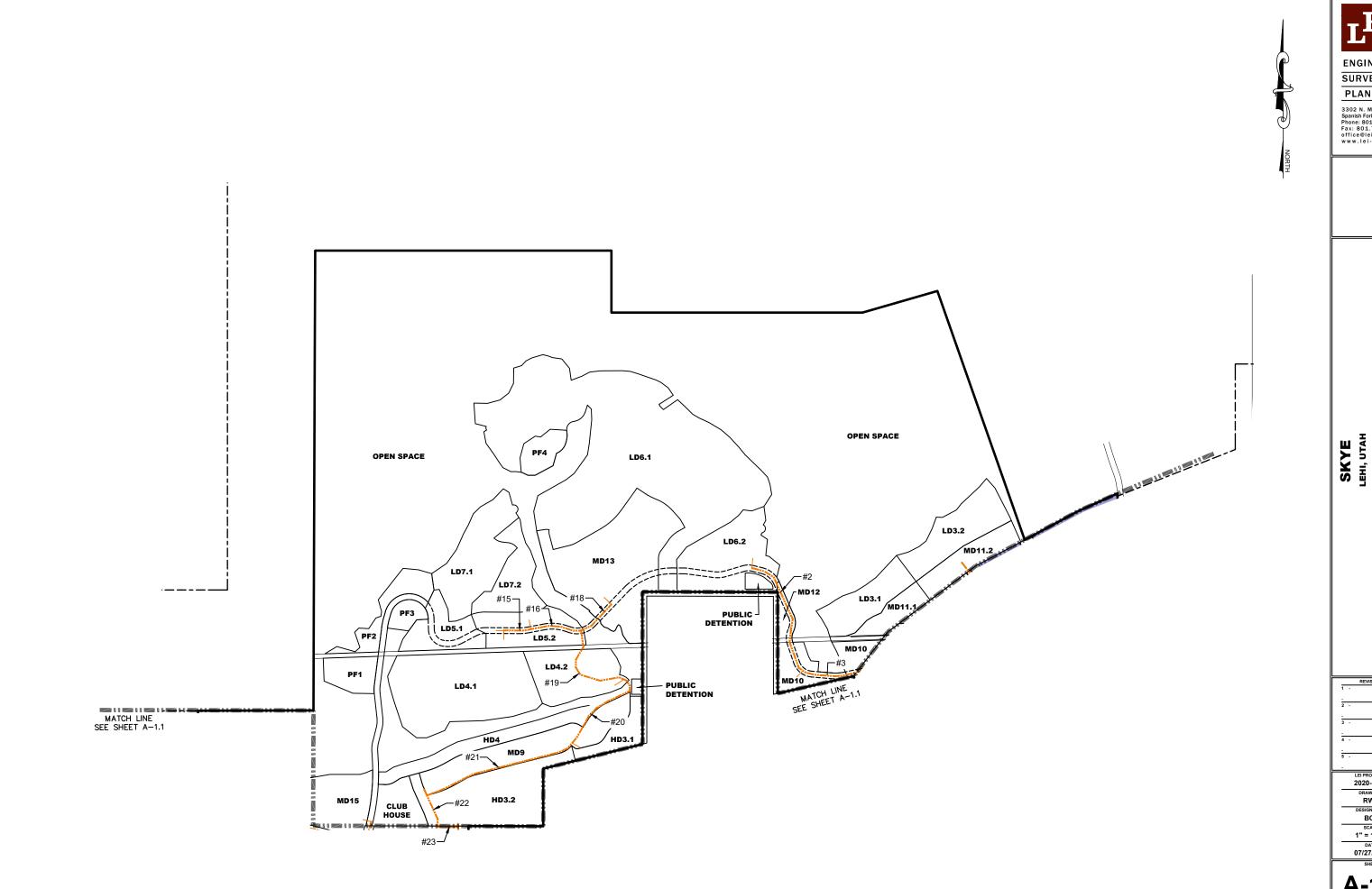
D. Slope

- 1. The pipe diameter and slope shall be selected to obtain velocities to minimize settling problems.
- 2. All sewers shall be designed and constructed to give mean velocities of not less than 2 feet per second (0.61 meter per second), when flowing full, based on Manning's formula using an n value of 0.013.
- 3. Sewers shall be laid with uniform slope between manholes.
- 4. Table R317-3-2.3(D)(4) shows the minimum slopes which shall be provided; however, slopes greater than these are desirable.

S-3

Appendix A: Pipe Location Exhibit







SURVEYORS **PLANNERS**

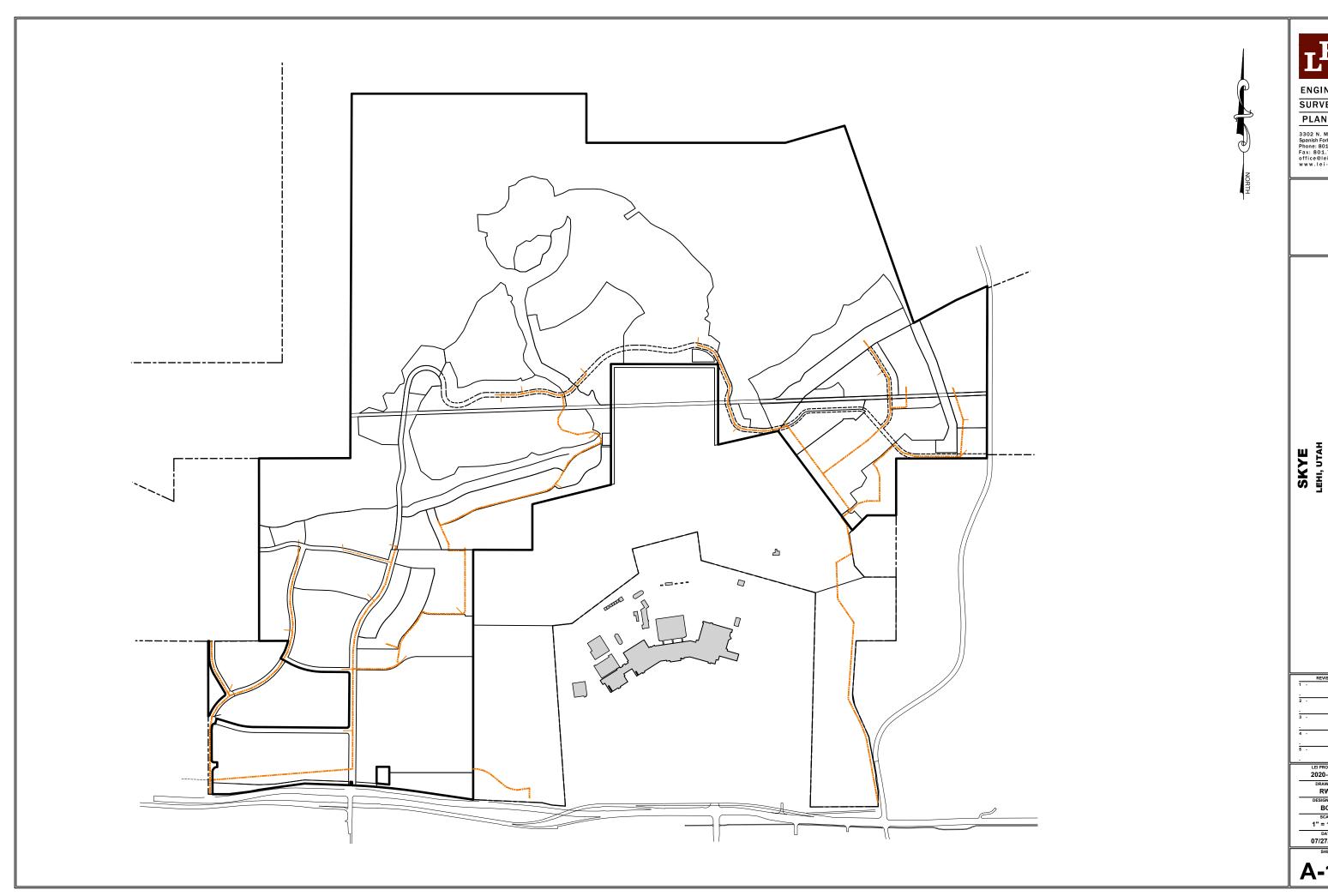
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SEWER PLAN

2020-0067 DRAWN BY: RWH

BCT SCALE: 1" = 1000'

DATE: 07/27/2023





ENGINEERS SURVEYORS PLANNERS

3302 N. Main Street Spanish Fork, UT 84660 Phone: 801.798.0555 Fax: 801.798.9393 office@lei-eng.com www.lei-eng.com

SEWER PLAN

LEI PROJECT #:
2020-0067
DRAWN BY:
RWH
DESIGNED BY:
BCT
SCALE:
1" = 1000'

DATE: 07/27/2023

A-1.3

Appendix B: Detailed Pipe Flows and Pipe Sizing

SKYE AREA PLAN

Sewer Flows - West Side A

Land Use	Area	Residential Units or ERU's	Capita/ERU	Capita	Flow/Capita	Average Flow	Peak Factor	Peak Hou	ly Flow	
	(acres)	(units)	(capita/unit)	(capita)	(gpd/capita)	(gpd)		(gpd)	(gpm)	
Mixed Use	106.29	749	3.8	2,846	100	284,600	3.0	853,800	593	
Civic	22.59	27	3.8	103	100	10,300	3.0	30,900	21	
LD	188.05	282	3.8	1,073	100	107,300	3.0	321,900	224	
HD	54.75	516	3.8	1,961	100	196,100	3.0	588,300	409	
MD	25.18	86	3.8	326	100	32,600	3.0	97,800	68	
MD	78.47	347	3.8	1,319	100	131,900	3.0	395,700	275	
School	42.98	24	3.8	91	100	9,100	3.0	27,300	19	
Clubhouse	5.97	6	3.8	22	100	2,200	3.0	6,600	5	
Offsite 1	290.18	558	3.8	2,120	100	212,040	3.0	636,120	442	
Total West Side A		2,595		9,861		986,140		2,958,420	2,054	

Sewer Flows - East Side B

	Land Use	Area	Residential Units or ERU's	Capita/ERU	Capita	Flow/Capita	Average Flow	Peak Factor	Peak Hou	rly Flow
		(acres)	(units)	(capita/unit)	(capita)	(gpd/capita)	(gpd)		(gpd)	(gpm)
ſ	AA	41.03	231	3.8	878	100	87,800	3.0	263,400	183
	Civic	4.44	2	3.8	8	100	800	3.0	2,400	2
I	LD	113.98	170	3.8	646	100	64,600	3.0	193,800	135
	VLD	15.22	18	3.8	68	100	6,800	3.0	20,400	14
	MD	7.99	48	3.8	182	100	18,200	3.0	54,600	38
	MD	40.99	152	3.8	578	100	57,800	3.0	173,400	120
-	Total East Side B		621		2,360		236,000		708,000	492

Sewer Flows - West Side C

_	Sewel Hows - West Side C									
	Land Use	Area	Residential Units or ERU's	Capita/ERU	Capita	Flow/Capita	Average Flow	Peak Factor	Peak Hou	rly Flow
		(acres)	(units)	(capita/unit)	(capita)	(gpd/capita)	(gpd)		(gpd)	(gpm)
	Mixed Use	9.70	78	3.8	296	100	29,600	3.0	88,800	62
	Total West Side C		78		296		29.600		88.800	62

Sewer Flows - Offsite Detailed

Ī	Offsite Area	Land Use	Area	Residential Units or ERU's	Capita/ERU	Capita	Flow/Capita	Average Flow	Peak Factor	Peak Hou	,
L			(acres)	(units)	(capita/unit)	(capita)	(gpd/capita)	(gpd)		(gpd)	(gpm)
	Offsite 1	Canyon Hills (Existing)	78.67	244	3.8	927	100	92,720	3.0	278,160	193
		Peck Property (Lehi - 1.5 units/ac)	58.89	88	3.8	334	100	33,440	3.0	100,320	70
		Entrata (Draper - 1.5 units/ac))	149.28	224	3.8	851	100	85,120	3.0	255,360	177
		Canyon Hills Church	3.34	2	3.8	8	100	760	3.0	2,280	2
	Total Offsite		_	558		2,120		212,040		636,120	442

Skye Area Plan Sewer Pipe Sizing

Pipe ID.	Contributing	Peak Hourly Flow	Peak Hourly Flow	Pipe Dia.	Min Slope	Qpipe	Velocity	Pipe
	Area (ac)	(gpm)	(cfs)	(inches)	(%)	(cfs)	(fps)	Check
1	MU1.2 (600 E outfall)	62	0.14	8	0.34	0.71	2.02	OK
2	LD6.2	19	0.04	8	0.34	0.71	2.02	OK
3	LD6.2, MD12, MD10	36	0.08	8	0.34	0.71	2.02	OK
4	LD6.2, MD12, MD10, LD3.1, MD11.1, MD5, MD6, MD7	126	0.28	8	0.34	0.71	2.02	OK
5	LD3.2, MD11.2	47	0.10	8	0.34	0.71	2.02	OK
6	LD3.2, MD11.2, MD8	86	0.19	8	0.34	0.71	2.02	OK
7	AA1	49	0.11	8	0.34	0.71	2.02	OK
8	AA1, MD4	59	0.13	8	0.34	0.71	2.02	OK
9	LD3.2, MD11.2, MD8, AA1, MD4	146	0.32	8	0.34	0.71	2.02	OK
10	LD6.2, MD12, MD10, LD3.1, MD11.1, MD5, MD6, MD7, LD3.2, MD11.2, MD8, AA1, MD4	272	0.61	8	0.34	0.71	2.02	OK
11	AA2	110	0.25	8	0.34	0.71	2.02	OK
12	AA2, AA3	134	0.30	8	0.34	0.71	2.02	OK
13	AA2, AA3, CIV 2	135	0.30	8	0.34	0.71	2.02	OK
14	Full Build-out (East Side)	492	1.10	8	0.82	1.10	3.14	ок
15	LD5.1, LD4.1, LD7.1	61	0.14	8	0.34	0.71	2.02	OK
16	LD5.1, LD4.1, LD7.1, LD7.2	73	0.16	8	0.34	0.71	2.02	OK
18	LD6.1, MD13	162	0.36	8	0.34	0.71	2.02	OK
19	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2	265	0.59	8	0.34	0.71	2.02	OK
20	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2	277	0.62	8	0.34	0.71	2.02	OK
21	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1	319	0.71	8	0.35	0.72	2.05	OK
22	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4	456	1.02	8	0.71	1.02	2.92	OK
23	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE	459	1.02	8	0.72	1.03	2.95	OK
24	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2	621	1.38	8	1.31	1.39	3.97	OK
24a	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2, HD1	748	1.67	8	1.89	1.67	4.77	OK
25	MD1. MD2	95	0.21	8	0.34	0.71	2.02	OK
26	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2, HD1, MD1, MD2	843	1.88	8	2.41	1.88	5.39	OK
27	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2, HD1, MD1, MD2, MU2	1088	2.42	12	0.47	2.45	3.12	OK
28	MD15	20	0.04	8	0.34	0.71	2.02	OK
29	MD14	16	0.04	8	0.34	0.71	2.02	OK
30	MD15. MD16	39	0.09	8	0.34	0.71	2.02	OK
31	MD15. MD14. MD16. LD1	71	0.16	8	0.34	0.71	2.02	OK
32	LD5.1. LD4.1. LD7.1. LD7.2. LD6.1. MD13. LD5.2. LD4.2. HD3.1. MD9. HD4. CLUBHOUSE. HD3.2. HD1. MD1. MD2. MU2. MD15. MD14. MD16. LD1. CIV1	1181	2.63	12	0.55	2.65	3.37	OK
33	LD5.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2, HD1, MD1, MD2, MU2, MD15, MD15, MD16, MD16, LD7, CV1, SCH2	1193	2.66	12	0.56	2.67	3.40	OK
34	LDS.1, LD7.1, LD7.2, LD6.1, MD13, LD5.2, LD4.2, HD3.1, MD9, HD4, CLUBHOUSE, HD3.2, HD1, MD1, MD2, MU2, MD15, MD14, MD16, LD1, CV1, SCH2, MU1.1, MU1.2	1540	3.43	18	0.30	3.56	2.01	OK
35	LD3.1, LD4.1, LD7.1, LD7.2, LD6.1, MD13, LD3.2, LD4.2, HD5.1, MD9, HD4, CLUBHOUGE, HD3.2, HD1, MD1, MD2, MD13, MD14, MD16, LD1, G171, G171	20	0.04	8	0.11	0.71	2.02	OK
36	HD2. MD3	66	0.15	8	0.34	0.71	2.02	OK
37	OFFSITE 1	442	0.13	8	0.66	0.71	2.02	OK
38	HD2, MD3, OFFSITE 1, SCH1	514	1.15	12	0.00	1.60	2.02	OK
38 39	All Pipes on West Side	2054	4.58	18	0.20	4.59	2.03 2.60	OK OK
Note:	All Fipes oil West Side	2004	4.00	10	0.15	4.05	2.00	UK

Note

^{1.} Offsite 1 is composed of the following properties - Canyon Hills, Peck Property, Entrata, and Canyon Hills Church (See Sewer Flows table on previous page for a detailed breakdown of offsite flows)